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SYMMETRA LX UNIT

Structural Calculations For Seismic Anchorage

(also covers the following LX unit configurations: SYAF8KT, SYA4K8P, SYA8K8P, SYAF16KT, SYA8K16P, SYA12K16P, SYA16K16P, SYAF16KXR9T, SYA8K16PXR, SYA12K16PXR, SYA16K16PXR, SYAXR9B9, SYBFXR9)

Prepared for:
American Power Conversion

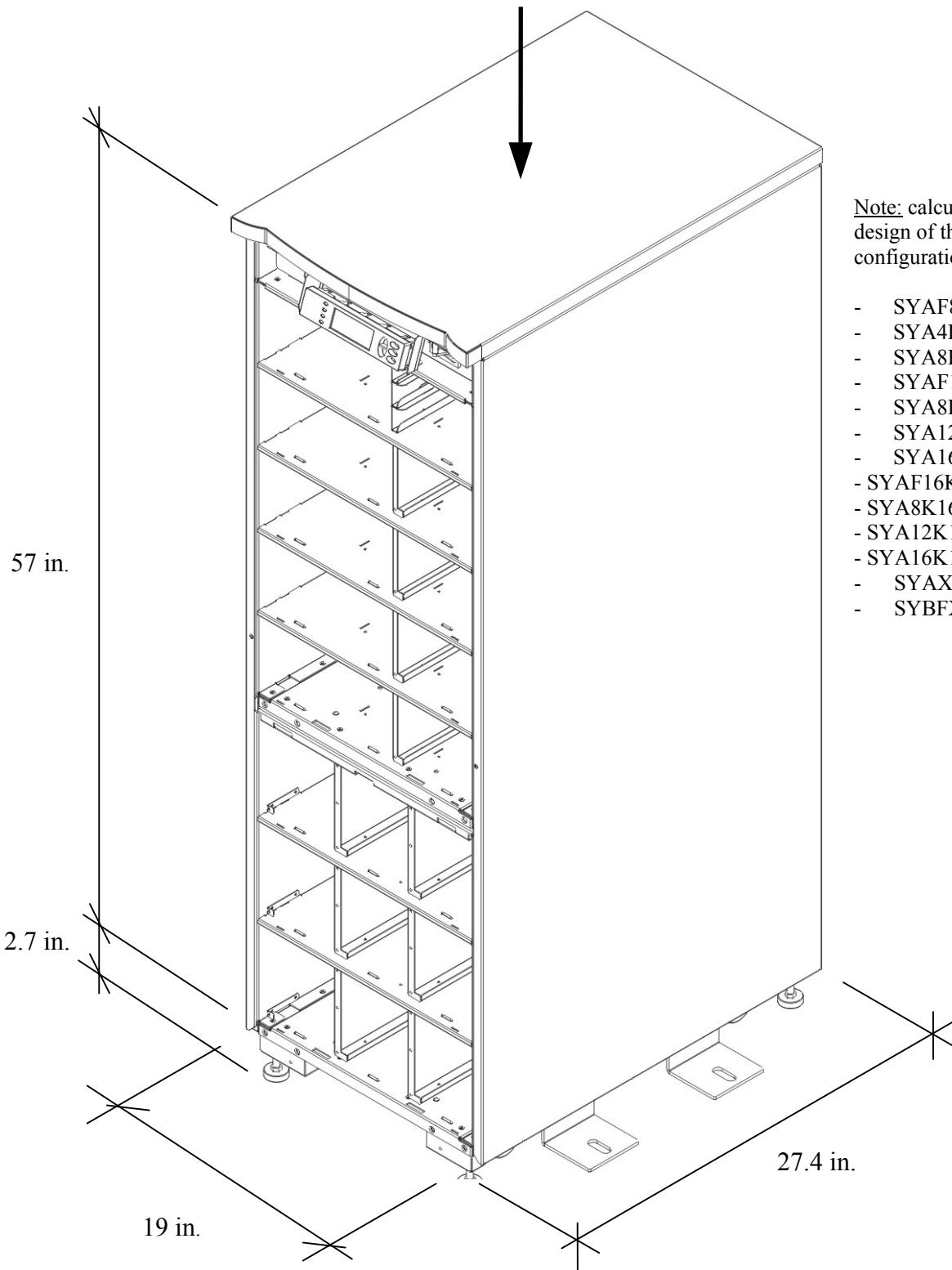
October 15, 2004
RMJ Job No. 04272

103 Linden Avenue
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(650) 871-2282 FAX (650) 871-2459



MODEL: SYMMETRA LX UNIT
(fully loaded w/ 5 power modules &
13 Battery Modules, governs design)

Dead Load = 400 lbs
Live Load = 997 lbs



Note: calculations also apply to design of the following LX configurations:

- SYAF8KT = 217LBS
- SYA4K8P = 314LBS
- SYA8K8P = 411LBS
- SYAF16KT = 256LBS
- SYA8K16P = 450LBS
- SYA12K16P = 547LBS
- SYA16K16P = 644LBS
- SYAF16KXR9T = 400LBS
- SYA8K16PXR = 1042LBS
- SYA12K16PXR = 1075LBS
- SYA16K16PXR = 1108LBS
- SYAXR9B9 = 768LBS
- SYBFXR9 = 192LBS

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SYMMETRA LX UNIT
APC
Job No. 04272
By: AM
Date: 10/15/04
Page: 2

19" x 27.4" x 59.7" SYMMETRA LX UNIT

Seismic Stability

Load Case: Fully Loaded: w/ 5 power modules and 13 battery modules (Anchorage at ground floor)

Unit		Center of Mass-Unit			Center of Mass-Contents	
Unit	Part	Weight (lbs)	X (in)	Y (in)	Z (in)	
Sym. LX	Frame	400	9.37	13.6	32.8	$C_a = 0.66$ (Zone 4, $N_a=1.5$) $I_p = 1$ (Importance factor) $a_p = 1$ (Cabinets) $R_p = 3$ (Cabinets) $h_s/h_r = 0$ (Floor) calc. at ground floor $W = 1397$ lbs
	Max Content Wt.	997	8.85	11.8	26.2	$F_p = 0.16 W = 220$ lbs $F_{pmin} = 0.33 W = 461$ lbs $F_{pmax} = 1.89 W = 2634$ lbs $F_p = 0.33 W = 461$ lbs
<u>Longitudinal Stability</u>		<u>Anchorage Force (lbs)</u>				
Overturning moment=	12950 in-lbs	76	total			
0.9 X Resisting moment=	11314 in-lbs					
<u>Transverse Stability</u>		<u>Anchorage Force (lbs)</u>				
Overturning moment=	12950 in-lbs	Stable, No Anchorage Required				
0.9 X Resisting moment=	15484 in-lbs					

1997 Uniform Building Code

Section 1632 - Lateral Force on Elements of Structures, Nonstructural Components and Equipment Supported by Structures

The total design lateral seismic force, F_p , shall be determined from the following formula:

$$F_p = 4.0 C_a I_p W_p$$

Alternatively, F_p may be calculated using the following formula:

$$F_p = a_p C_a I_p (1 + 3 \cdot h_s / h_r) W_p / R_p$$

Except that:

F_p shall not be less than $0.7 C_a I_p W_p$ and
need not be more than $4 C_a I_p W_p$

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19" x 27.4" x 59.7" SYMMETRA LX UNIT

Seismic Stability - Anchorage at Ground Floor

ANCHORAGE

CONDITION	ANCHORAGE FORCE	ANCHORS	CAPACITY	SCREWS	CAPACITY
SINGLE UNIT	76/2=38 lb	1-3/8"φ	784 lb	2-#12	746 lb
GANGED UNITS	---	1-3/8"φ	784 lb	2-#12	746 lb

CHECK COMBINED SHEAR/TENSION

$$\begin{array}{lll} P_{\max} = & 38 \text{ lb} & V_{\max} = 461/4 \text{ bolts} = & 115.25 \text{ lb} \\ P_{\text{allow}} = & 784 \text{ lb} & V_{\text{allow}} = & 1018 \text{ lb} \end{array}$$

$$(38/784)^{(5/3)} + (115.25/1018)^{(5/3)} = 0.03 < 1 \text{ OK}$$

ANCHOR CAPACITIES

3/8"φ Red Head Trubolt Wedge Anchors (ICBO Report No. 1372)
Embedded 3 " into 3000 psi Reg. Wt. Concrete

1-3/8"φ:	588 x 1.33= 784 lb	Capacity
2-3/8"φ:	spaced at 6 " = 784 x 2= 1568 lb	
2-3/8"φ:	spaced at 4 " = 784 x 2 x 0.8= 1294 lb	
2-3/8"φ:	spaced at 3 " = 784 x 2 x 0.7= 1098 lb	
3-3/8"φ:	spaced at 6 " = 784 x 3= 2352 lb	
#12 Sheet metal screw into 18 GA. Min (SSMA / ICBO No. 4943P) =	280 x 1.33= 373 lb	

- ∴ Use 3/8" φ "Red Head" Trubolt Wedge Anchors at +/- 21.6" o.c. for single unit.
For ganged units use 3/8" φ "Red Head" Trubolt Wedge Anchors at +/- 43.2" o.c.
max. as shown on installation details.

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SYMMETRA LX UNIT
APC
Job No. 04272
By: AM
Date: 10/15/04
Page: 4

19" x 27.4" x 59.7" SYMMETRA LX UNIT

Seismic Stability

Load Case: Fully Loaded: w/5 power modules and 13 battery modules (Anchorage at Second Floor or Above)

Unit		Center of Mass-Unit			Center of Mass-Contents	
Length=	19 inches	Length=	9.63 inches		Length=	10.15 inches
Width=	27.4 inches	Width=	13.6 inches		Width=	11.8 inches
Height=	59.7 inches	Height=	32.8 inches		Height=	26.2 inches
<u>Unit</u>			<u>Seismic Force (See bottom of page)</u>			
Sym. RT	Part	Weight (lbs)	X (in)	Y (in)	Z (in)	
	Frame	400	9.37	13.6	32.8	$C_a = 0.66$ (Zone 4, $N_a=1.5$) $I_p = 1$ (Importance factor) $a_p = 1$ (Cabinets) $R_p = 3$ (Cabinets) $h_r/h_c = 1$ (Floor) \leftarrow calc. at roof $W = 1397$ lbs
Max Content Wt.		997	8.85	11.8	26.2	$F_p = 0.63 W = 878$ lbs $F_{pmin} = 0.33 W = 461$ lbs $F_{pmax} = 1.89 W = 2634$ lbs $F_p = 0.63 W = 878$ lbs
<u>Longitudinal Anchorage Spacing</u> =			21.6 in.	<u>Anchorage Force (lbs)</u>		
Transverse Anchorage Spacing=			10 in.	618	total	

Longitudinal Stability

Oversetting moment= 24666 in-lbs
0.9 X Resisting moment= 11314 in-lbs

Anchorage Force (lbs)

618 total

Transverse Stability

Oversetting moment= 24666 in-lbs
0.9 X Resisting moment= 15484 in-lbs

Anchorage Force (lbs)

918 total

1997 Uniform Building Code

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$$F_p = a_p C_a I_p (1 + 3 \cdot h_r / h_c) W_p / R_p$$

Except that:

F_p shall not be less than $0.7 C_a I_p W_p$ and
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19" x 27.4" x 59.7" SYMMETRA LX UNIT

Seismic Stability - Anchorage at 2nd Floor & Above

ANCHORAGE

CONDITION	ANCHORAGE FORCE	ANCHORS	CAPACITY	SCREWS	CAPACITY
SINGLE UNIT	918/2=459 lb	1-3/8"φ	784 lb	2-#12	746 lb
GANGED UNITS	---	1-3/8"φ	784 lb	2-#12	746 lb

CHECK COMBINED SHEAR/TENSION

$$\begin{array}{ll} P_{\max} = & 459 \text{ lb} \\ P_{\text{allow}} = & 784 \text{ lb} \end{array} \quad \begin{array}{ll} V_{\max} = 878/4 \text{ bolts} = & 219.5 \text{ lb} \\ V_{\text{allow}} = & 1018 \text{ lb} \end{array}$$

$$(459/784)^{(5/3)} + (219.5/1018)^{(5/3)} = 0.49 < 1 \text{ OK}$$

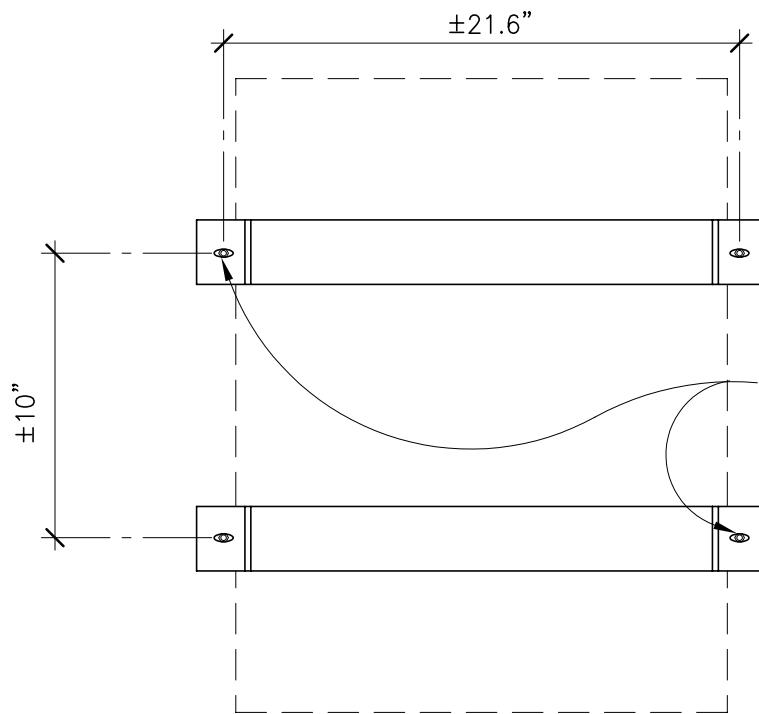
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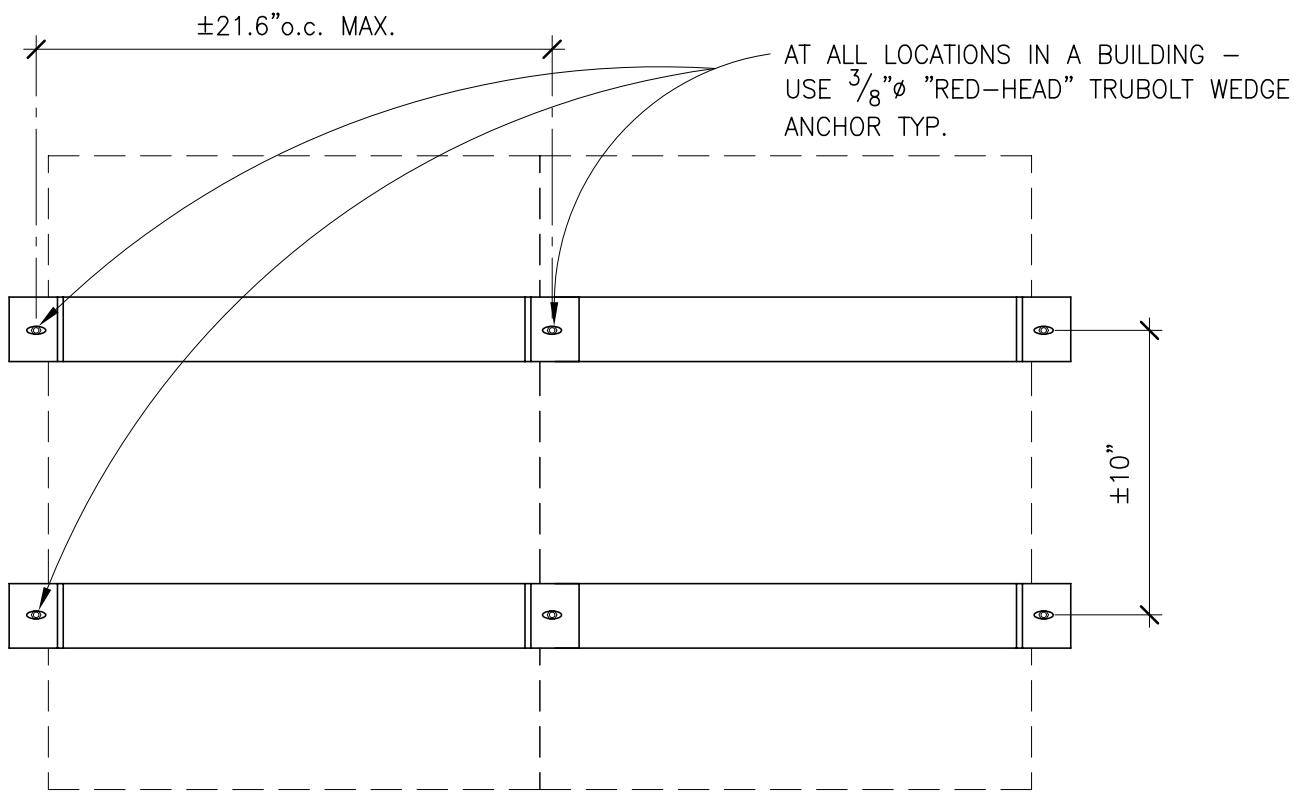
1-3/8"φ: 588 x 1.33= 784 lb
2-3/8"φ: spaced at 6 " = 784 x 2= 1568 lb
2-3/8"φ: spaced at 4 " = 784 x 2 x 0.8= 1294 lb
2-3/8"φ: spaced at 3 " = 784 x 2 x 0.7= 1098 lb
3-3/8"φ: spaced at 6 " = 784 x 3= 2352 lb

#12 Sheet metal screw into 18 GA. Min (SSMA / ICBO No. 4943P) = Capacity
280 x 1.33= 373 lb

- ∴ Use 3/8" φ "Red Head" Trubolt Wedge Anchors at +/- 21.6" o.c. for single unit.
For ganged units use 3/8" φ "Red Head" Trubolt Wedge Anchors at +/- 43.2" o.c.
max. as shown on installation details.

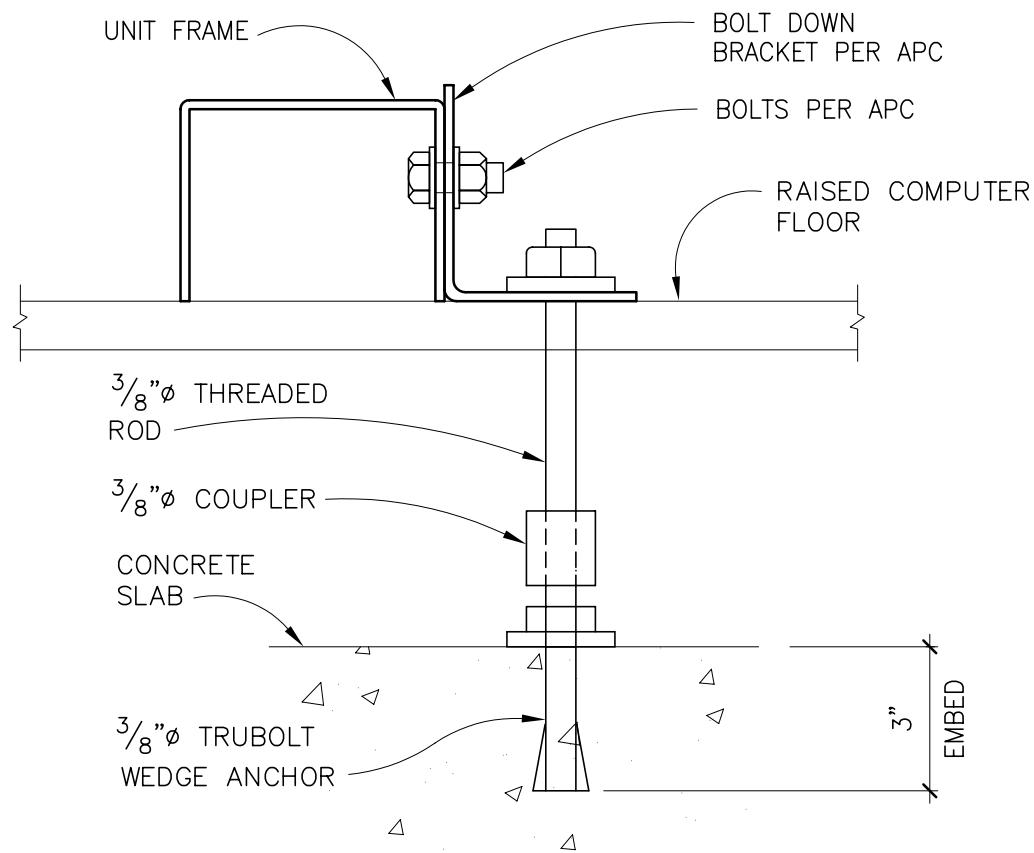


SINGLE UNIT PLAN



GANGED UNIT PLAN

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		Signed by	AM	Date	
				10/2004	



ANCHORAGE AT
RAISED FLOOR

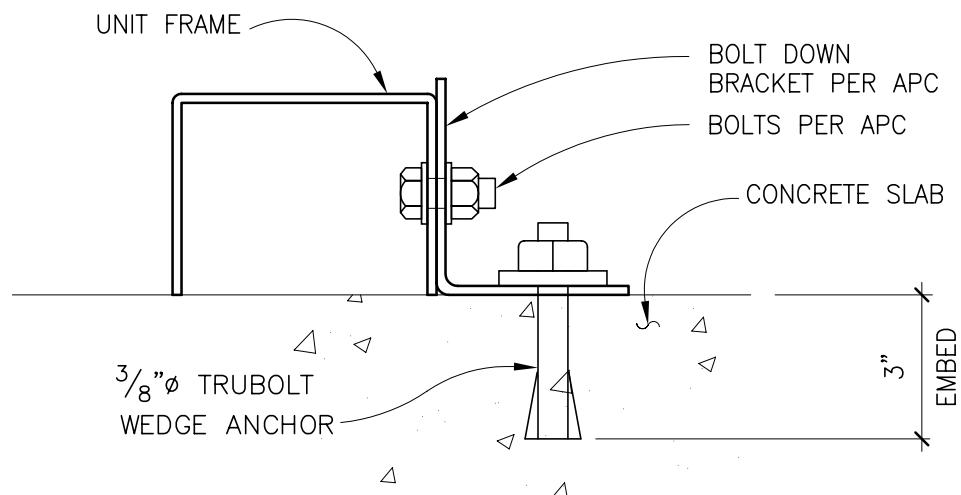


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Job No.	04272
Sheet No.	2
OF	3



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